



Rehabilitation of Historical Steel Bridge No. 321 over Rio Grande de Manatí

ACT Contract No. 668504

Puente Mata de Plátano / Ing. Juan José Jiménez

March 31, 2017
Alvin Rodríguez, PE
Elvin Pérez, PE

Ciales, PR

OBJECTIVES

- ❑ Team Information
- ❑ Location
- ❑ Historical Relevance
- ❑ Bridge Description
- ❑ Analysis of Alternatives
- ❑ Rehabilitation Considerations
 - Design guidelines exceptions
 - Bridge capacity
 - Rehabilitation schemes
- ❑ Highlights

REHABILITATION OF HISTORICAL STEEL BRIDGE NO. 321

TEAM INFORMATION

☐ Owner

Puerto Rico Highway & Transportation Authority (PRHTA)

☐ Planning, Design & Services During Construction

CMA Architects & Engineers LLC, PR
Sparks Engineering Consultant, TX

☐ Inspection & Safety

Puerto Rico Highway & Transportation Authority (PRHTA)

☐ Contractor

CD Builders - General Contractor
Cholo Onsite - Structural Steel Contractor

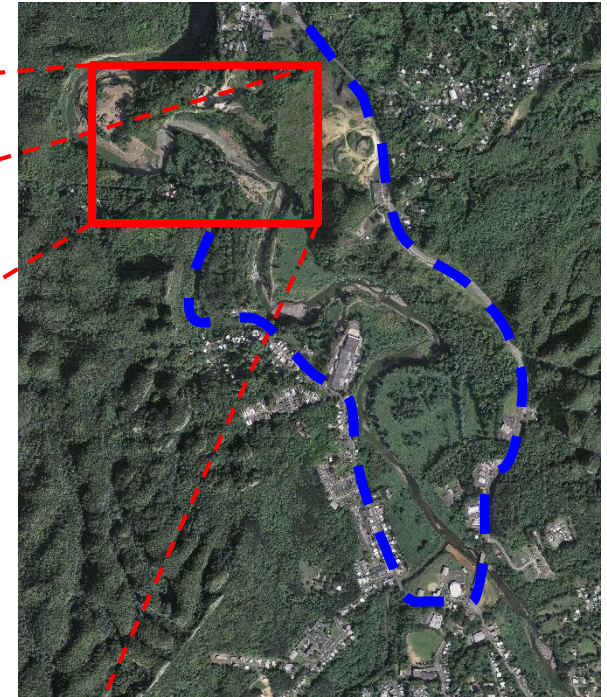
REHABILITATION OF HISTORICAL STEEL BRIDGE NO. 321

LOCATION

- **PR-6685** Km. 9.7 over *Rio Grande de Manati*
Municipality of Ciales-Manati



Manati, PR



Ciales, PR

HISTORICAL RELEVANCE

❑ Centenary Bridge

- Erected in 1905 by Eng. Luis Ninlliat
- Erected during the American Military Government
- **112** year-old bridge

❑ Historic Bridge Registry

- State Historic Preservation Office **(SHPO)**
National Registry Nomination (1995)
 - ✓ **Design integrity**
Minor rehabilitation on structural elements
 - ✓ **Original Elements**
Workmanship and materials
 - ✓ **Locality**
Importance to the community

❑ One of a Kind

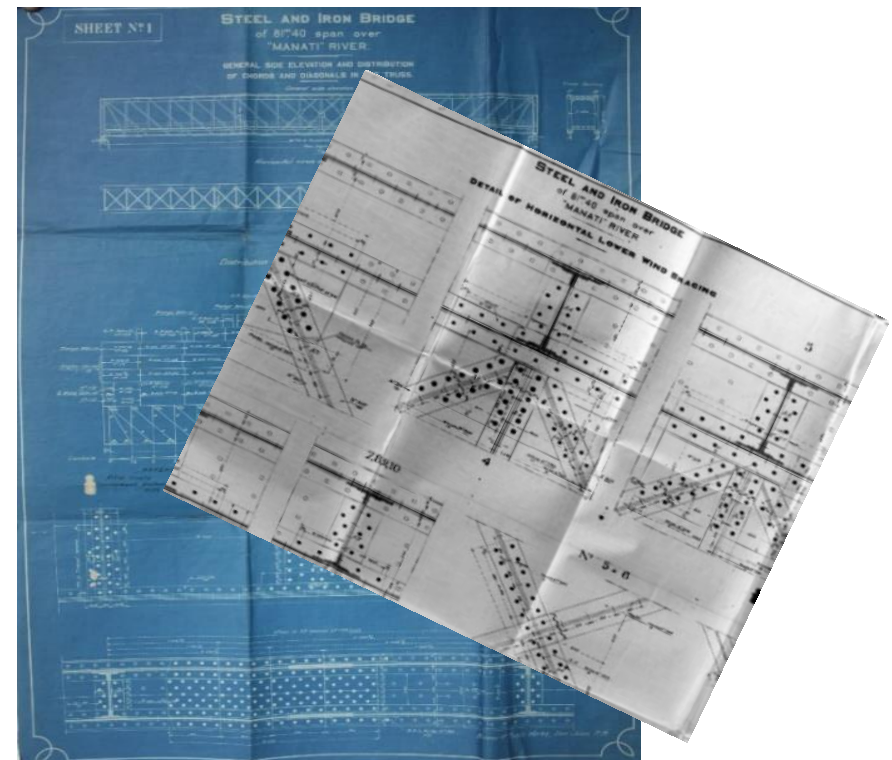
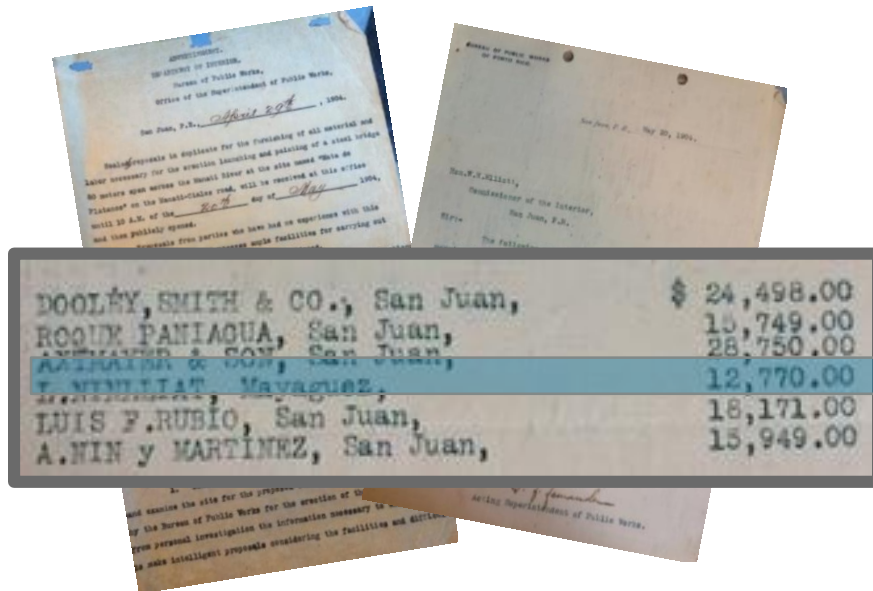
- Only **Double Whipple Riveted Truss** in PR

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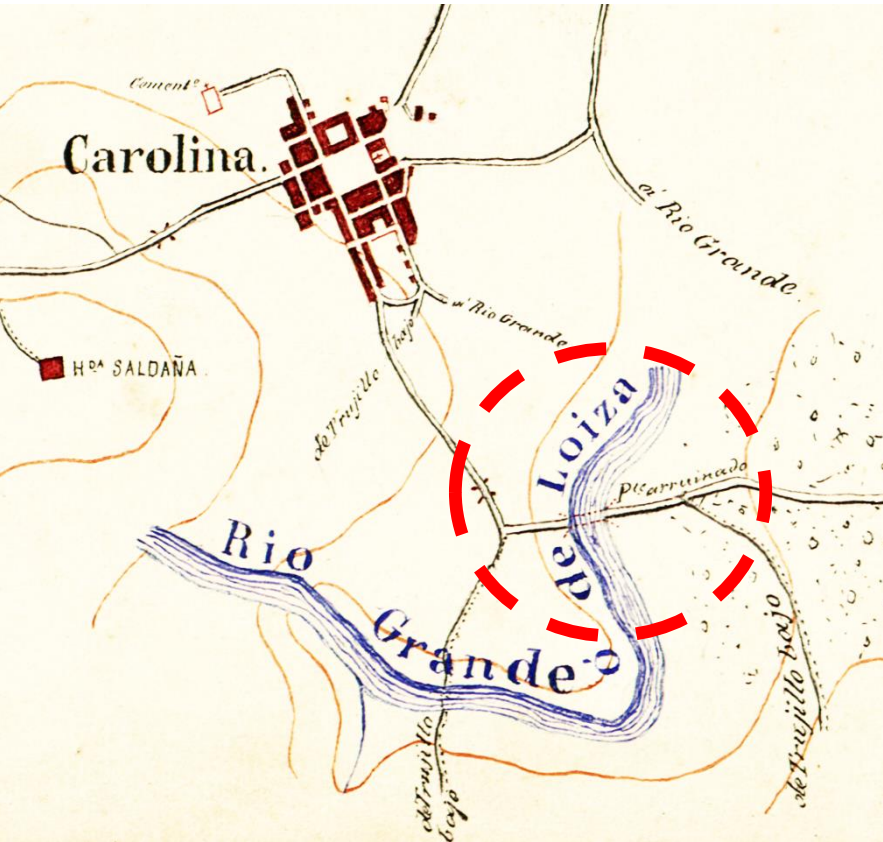
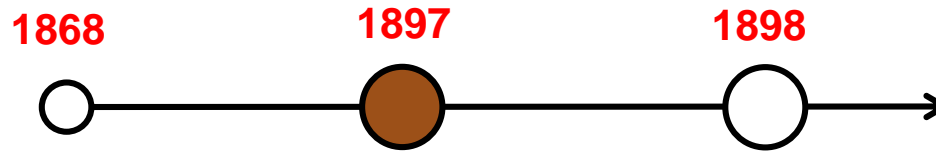
HISTORICAL RELEVANCE

❑ Bid Documents

- Erection & Painting
- Bid / Due Date: April 29th 1904 / May 20th 1904
- Lowest Bidder: \$12,770.00 [**Approx. \$750k in 2017**]



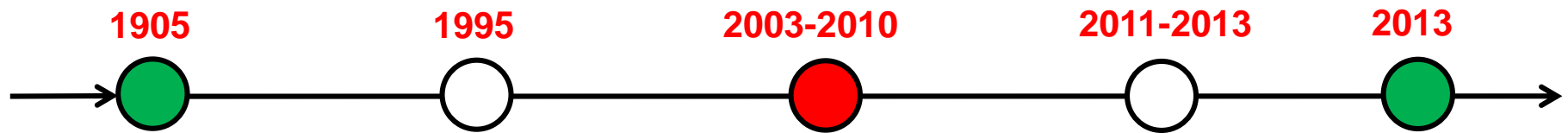
HISTORICAL RELEVANCE



- ❑ **June 1868**
 - Existing bridge in San Fernando de Carolina **swept away** during high floods
- ❑ **May 1897**
 - Municipality of Carolina requests to Spanish Government funds for a **new bridge**
- ❑ **November 1897**
 - Spanish Government **approves** a bridge over Rio Grande de Loiza [**Puente Principe de Asturias**]
- ❑ **March 1898**
 - **Foundation construction** commences in Carolina.

REHABILITATION OF HISTORICAL STEEL BRIDGE NO. 321

HISTORICAL RELEVANCE



□ April 1905

- American Military Government **determines to relocate Bridge** from Carolina to Ciales

□ June 1995

- Bridge is **included** in the **National Historical Registry**

□ January 2003-2010

- Evidence of **significant deterioration**
- Capacity Limited to 9 tons
- PRHTA recommended **Bridge Closure**

□ February 2011 - 2013

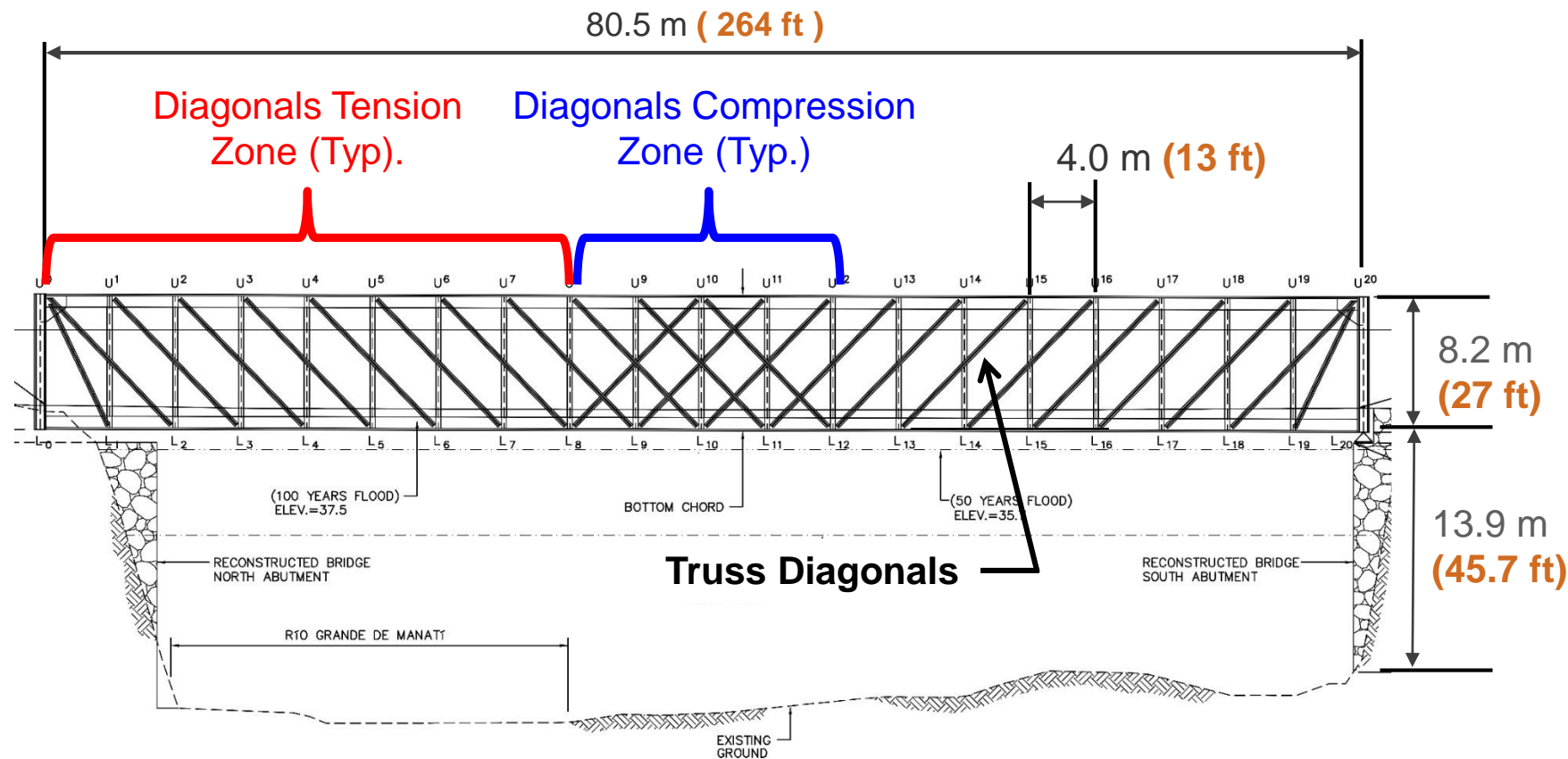
- Detailed bridge inspections & evaluations
- **Alternative evaluations**
- Community Meetings Coordination



Dedicated to Juan Jose Jimenez (1945) DTOP Superintendent

REHABILITATION OF HISTORICAL STEEL BRIDGE NO. 321

BRIDGE DESCRIPTION



ELEVATION

REHABILITATION OF HISTORICAL STEEL BRIDGE NO. 321

BRIDGE DESCRIPTION



Photo 2010

☐ Steel

- Built-up Sections
- Rivet connections
- Fy Min. 26ksi

☐ Roadway

- Asphalt
- Compacted soil base

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BRIDGE DESCRIPTION

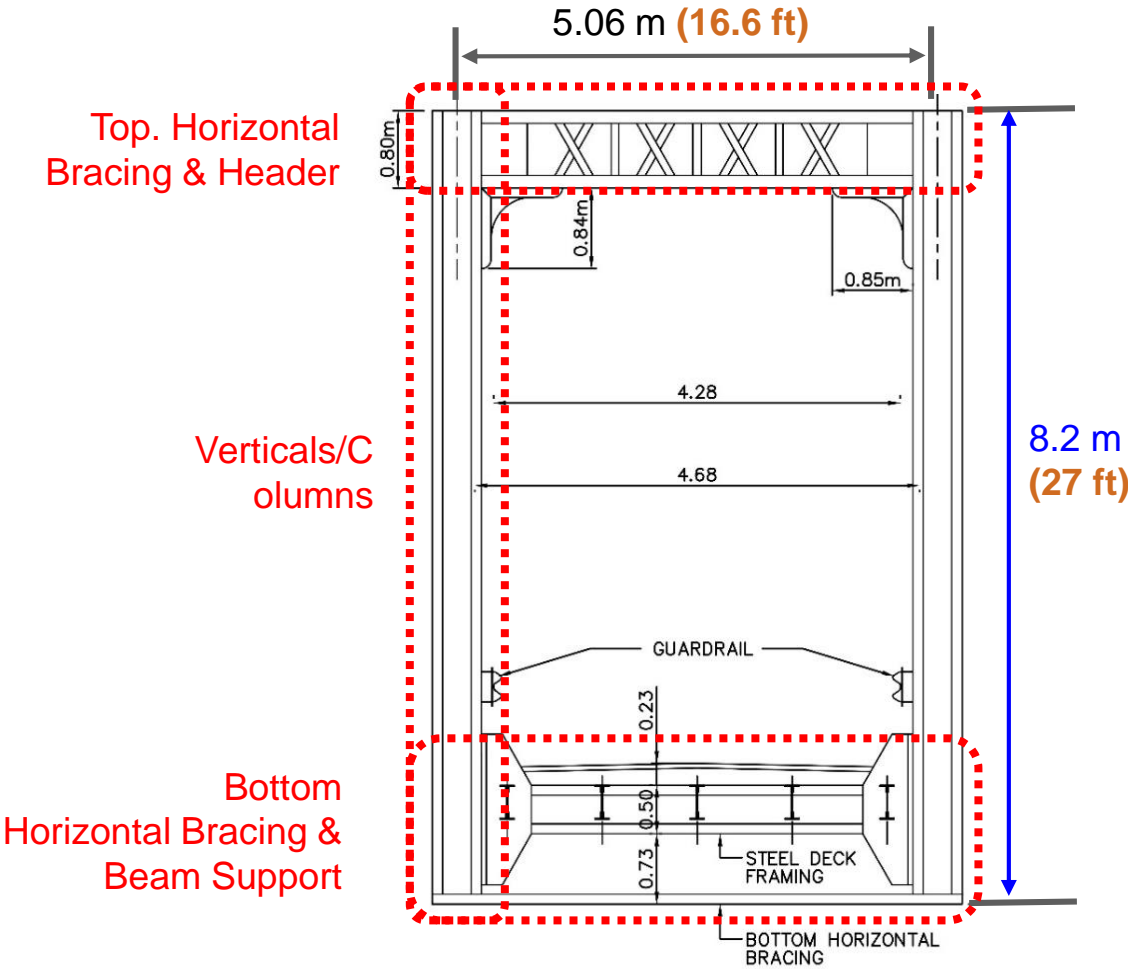


Photo 2012

TYPICAL INTERMEDIATE
FRAMES

REHABILITATION OF HISTORICAL STEEL BRIDGE NO. 321

ANALYSIS OF ALTERNATIVES

❑ Alternate No. 1:

No action | Permanent Bridge Closing

- Increases traffic on PR-149
- Increase in travel time from local commuters of nearby sector
- Increase in travel distance from point-of-origin to destination

❑ Alternate No. 2:

Replacement | In-kind Bridge \$3.8MM

- Single lane
- Slightly higher in elevation to be above 100yr flood level
- Adverse effect to the historical concept

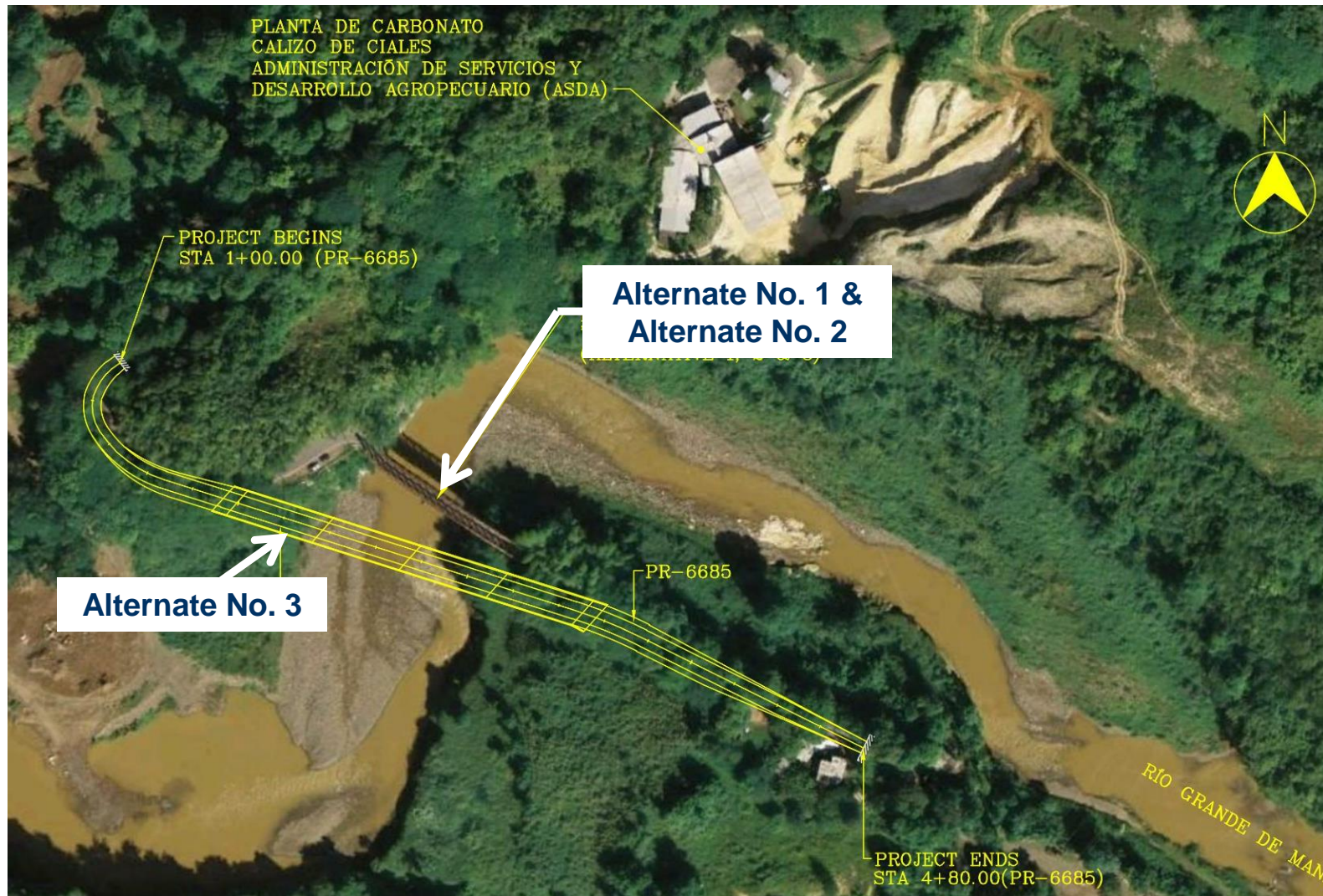
❑ Alternate No. 3:

Construction of New Bridge \$5.5MM

- Four spans: 38 m each / 152 m total span
- Overall dimension: Two 3.65m wide lanes / 2.40m shoulders
- No truck capacity limitations
- Removal of existing bridge due to potential collapse during 100yr flood level

REHABILITATION OF HISTORICAL STEEL BRIDGE NO. 321

ANALYSIS OF ALTERNATIVES



ANALYSIS OF ALTERNATIVES

Let's think out of the box....

*How about **rehabilitating the existing bridge?***

☐ Alternate No. 4:

Rehabilitation \$3.3MM

- Maintain **historical aspect** of the bridge
- Minimize traffic impact on PR-149 due to continuity of local commuters
- Reduce **travel distance** for nearby sectors
- Minimize **complexity of construction** in an environmental sensitive area

☐ Rehabilitation Considerations

- ✓ **PRHTA Design guidelines exceptions**
- ✓ **Determine bridge capacity**
- ✓ **Establish rehabilitation schemes**

REHABILITATION OF HISTORICAL STEEL BRIDGE NO. 321

REHABILITATION CONSIDERATIONS

Description	Design Guidelines		Exceptions
Design Speed	1979 PRHDM: AASHTO 2004:	25 mph 40 mph	15 mph posted at bridge crossing.
Bridge Width	1979 PRHDM: AASHTO 2004:	6.10 m (Travel way) 10.90m (Face-to-Face Railings) Full width of roadway approach	3.80 m Single Lane with Traffic Controller. 4.00 m face-to-face
Vertical Clearance	1979 PRHDM: AASHTO 2004:	4.40 m 4.30 m	Actual Height: 5.97m Limited to 2.40m at approach.
Structural Capacity	AASHTO 2004	HS-30	HS-15 Based on Existing Condition Assessment

❖ No exception taken:

- Horizontal alignment, super-elevation, vertical alignment, grade, stopping sight distance, cross-slope, lateral offset to obstruction

REHABILITATION CONSIDERATIONS

❑ Bridge Capacity | Design Guidelines

- The Manual for Bridge Evaluation 2nd Edition **(2013 Interims)**
 - ✓ Allowable Stress Methodology (LRFD optional, no preference)
 - ✓ Tension Elements
- American Institute of Steel Construction **(AISC 360-05)**
 - ✓ Compression Elements

Elements Evaluated

Bottom Chord, Top Chord, Verticals, Diagonals, Beams, Rivets

Limiting Mechanisms
Evaluated:

Tension
Compression **(Local & Flexural Buckling)**

Live load
(HS-15 & HS-20):

Truck Load **(Point Load)**
Lane Load **(Uniform & Point Load)**

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REHABILITATION CONSIDERATIONS

- ❑ **Fracture Critical Members (FCM)** - Tension elements whose failure would be expected to result in collapse of the bridge.

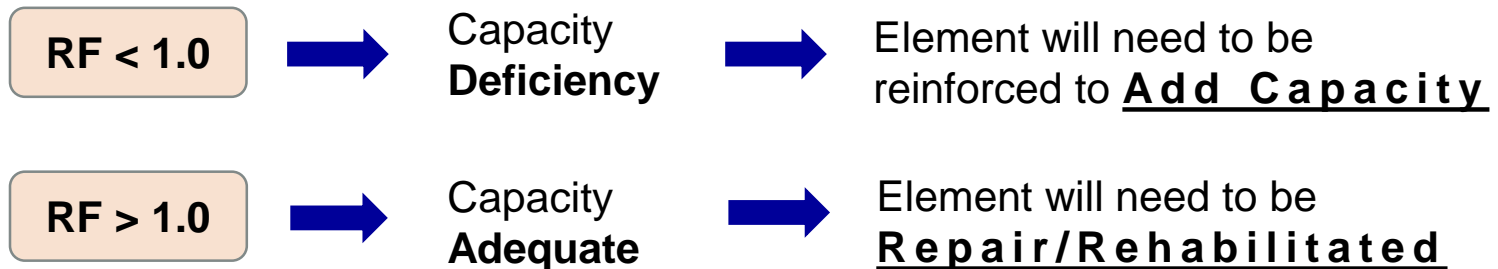


- **Load-redistribution** thru an all-riveted construction
- **Crack propagation** is not possible across adjacent elements
- **Redundancy** for the **pair of diagonal** truss elements

REHABILITATION CONSIDERATIONS

- ❑ **Load Rating Analysis (LRA)** - Calculations to determine the safe live load capacity of the bridge

$$\text{Rating Factor (RF)} = \frac{\text{Capacity} - \text{Dead Load}}{\text{Live Load Demand} \times \text{Impact Factor}}$$



- ❑ **Inventory Rating (IR)**

- ✓ Safe Live Load
- ✓ Indefinite vehicle use
- ✓ Minimum Maintenance

- ❑ **Operating Rating (OR)**

- ✓ Maximum live load permitted
- ✓ Limiting vehicle use
- ✓ Frequent Maintenance

REHABILITATION OF HISTORICAL STEEL BRIDGE NO. 321

REHABILITATION CONSIDERATIONS

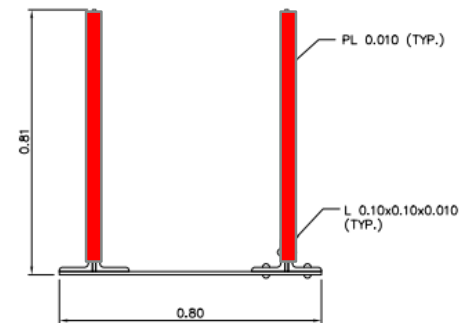
❑ Bottom Chords (Tension Element) - HS-15



- Perforation at bottom flange plate (Approx. 35% corrosion)
- Missing rivets
- Bottom chord to Vertical flange plate corrosion

RF = 0.50 (Inventory)
RF = 2.90 (Operating)

Element will need to be reinforced to **Add Capacity**



REHABILITATION OF HISTORICAL STEEL BRIDGE NO. 321

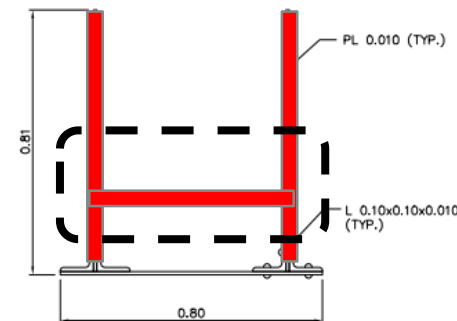
REHABILITATION CONSIDERATIONS

❑ Bottom Chords (Tension Element) – HS-15



- Additional plate added for Capacity
- Concrete added to provide drainage slopes

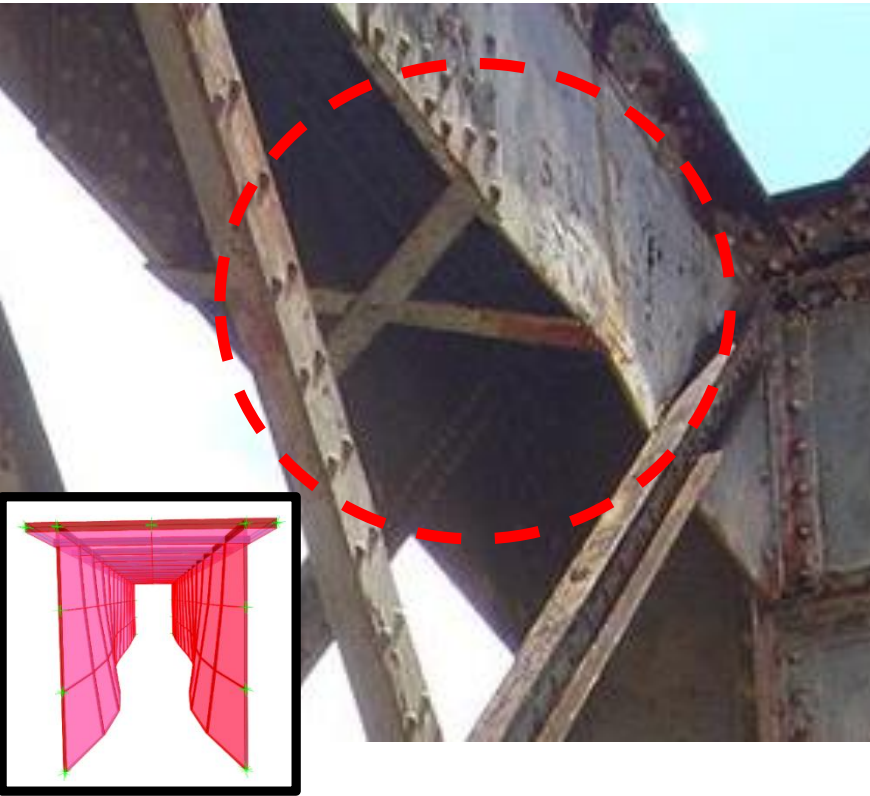
RF = 3.84 (Inventory)
RF = 7.46 (Operating)



REHABILITATION OF HISTORICAL STEEL BRIDGE NO. 321

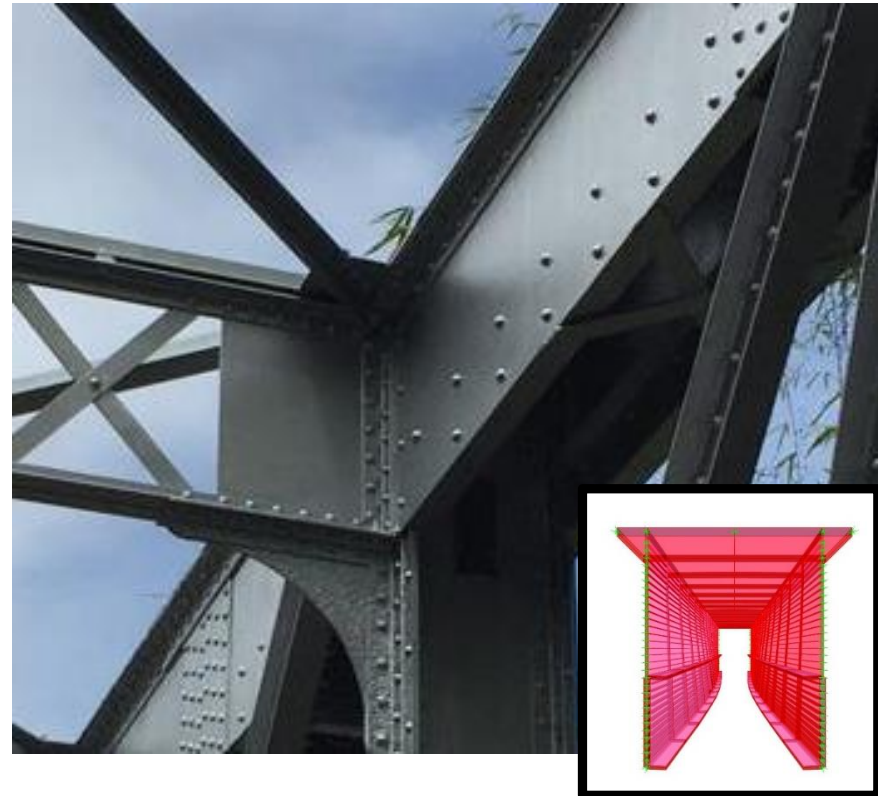
REHABILITATION CONSIDERATIONS

❑ Top Chords (Compression Element) – HS-15



- Top chord flange buckling
- Loss of section observed

RF = 0.57 (Inventory)
RF = 0.59 (Operating)



- Added new C12x30 channels each side for stiffness

RF = 7.50 (Inventory)
RF = 6.94 (Operating)

REHABILITATION OF HISTORICAL STEEL BRIDGE NO. 321

REHABILITATION CONSIDERATIONS

❑ East & West Truss Verticals – HS-15



- Severe Corrosion at bottom chord
- Missing rivets
- Deformation/bends at column flanges

RF = 7.07 (Inventory)
RF = 9.92 (Operating)

Element will need to be
Repair/Rehabilitated

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REHABILITATION CONSIDERATIONS

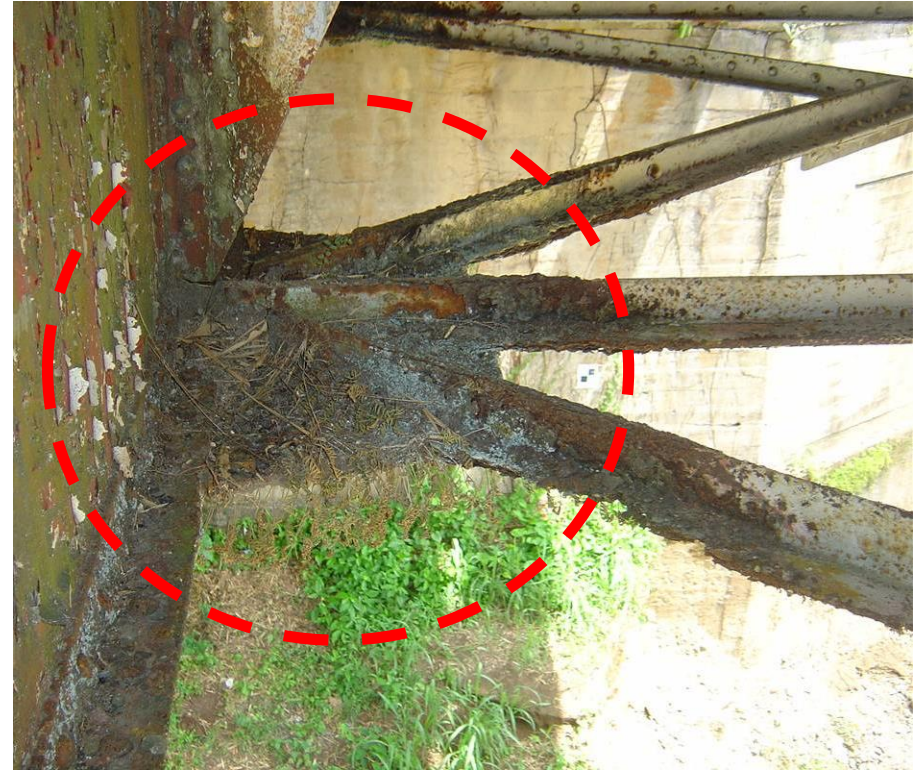
□ East & West Truss Vertical – HS-15



- Added new angles/plate bars & rivets replacement
- Added new transfer plates for lateral load transfer

REHABILITATION CONSIDERATIONS

❑ Horizontal Bottom Truss Diagonals

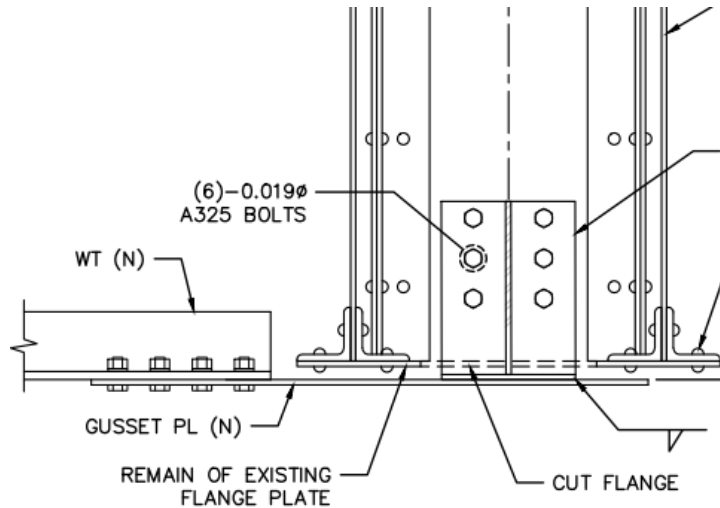


- Bottom truss with severe corrosion at angles and gusset plates
- Areas with significant loss of section
- Connection to column is compromised

REHABILITATION OF HISTORICAL STEEL BRIDGE NO. 321

REHABILITATION CONSIDERATIONS

❑ Horizontal Bottom Truss Diagonals



- Steel angle replacement
- Steel plate connection to column replaced

REHABILITATION OF HISTORICAL STEEL BRIDGE NO. 321

REHABILITATION CONSIDERATIONS

❑ Horizontal Top Truss Diagonals



- Top truss with severe corrosion at angles & gusset plates
- Local areas with significant loss of section

REHABILITATION OF HISTORICAL STEEL BRIDGE NO. 321

REHABILITATION CONSIDERATIONS

❑ Cross-Beams & Horizontal Truss Diagonals



- Replace top cross-beam
- Replace horizontal truss diagonals



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REHABILITATION CONSIDERATIONS

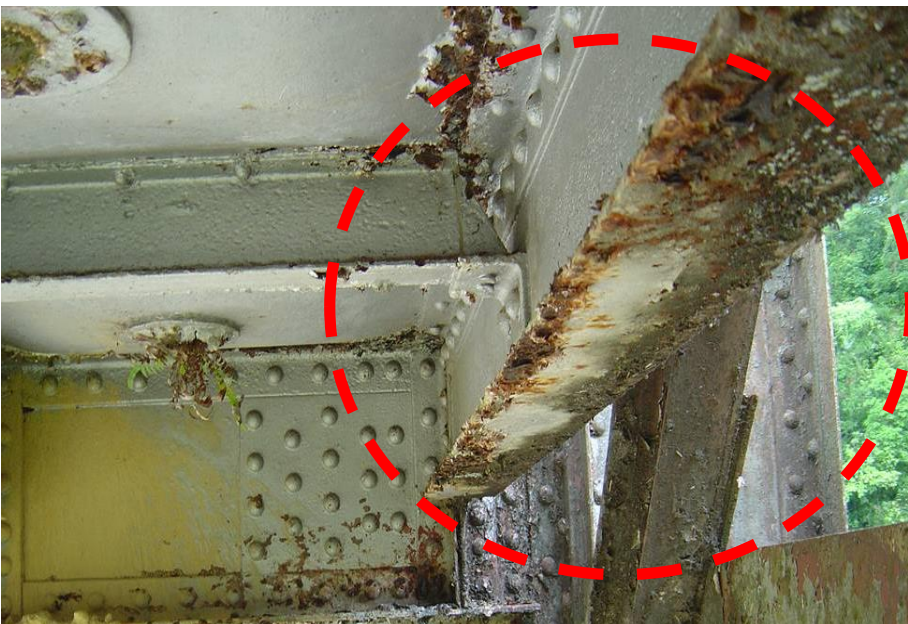
- ❑ Horizontal Top Truss Diagonals



REHABILITATION OF HISTORICAL STEEL BRIDGE NO. 321

REHABILITATION CONSIDERATIONS

□ Deck & Supporting Beam - (HS-15)



- Moderate underside corrosion
- Local areas with significant loss of section



- Replacement of angles

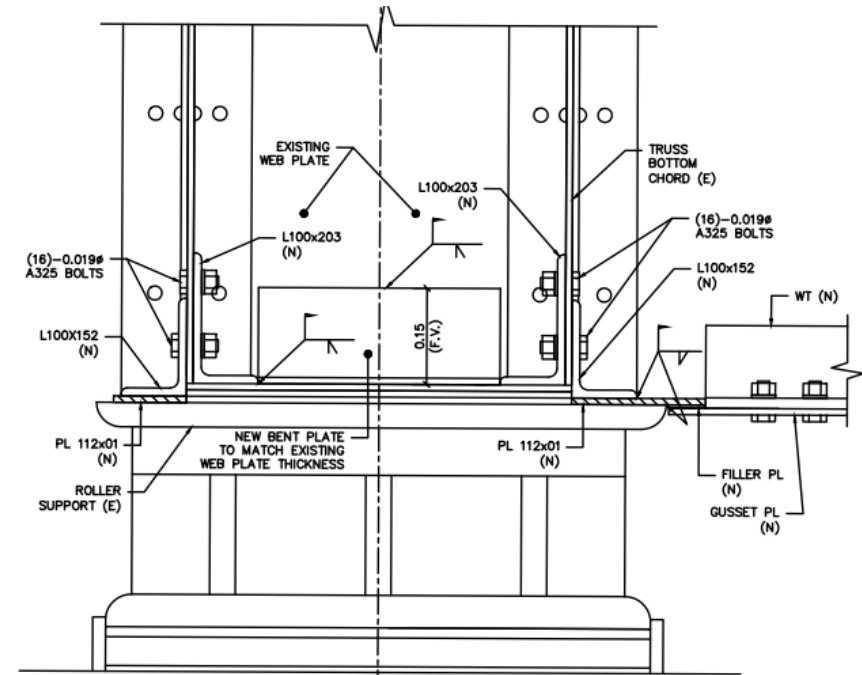
RF = 1.40 (Inventory)
RF = 2.30 (Operating)

Element will need to be
Repair/Rehabilitated

REHABILITATION OF HISTORICAL STEEL BRIDGE NO. 321

REHABILITATION CONSIDERATIONS

❑ Abutment

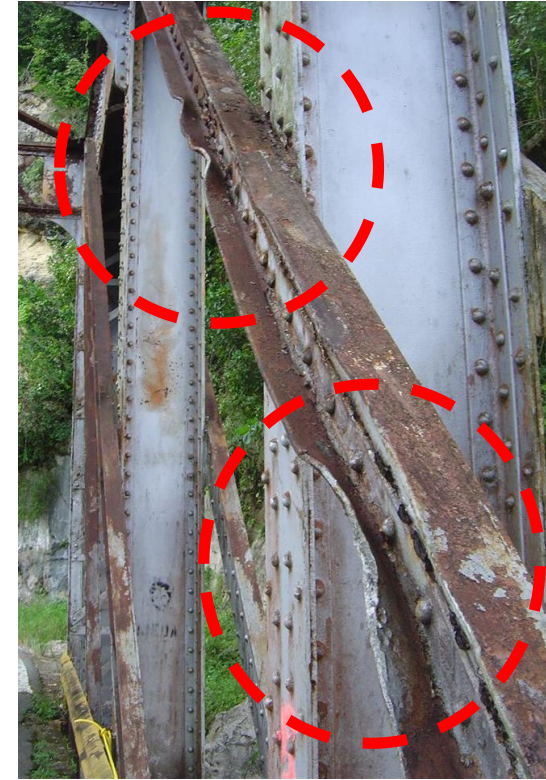


- Moderate corrosion at beam connection to roller
- Free sliding inhibited due to rust

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REHABILITATION CONSIDERATIONS

❑ East & West Truss Diagonals - (HS-15)



- Flange deformation/bends
- Significant corrosion

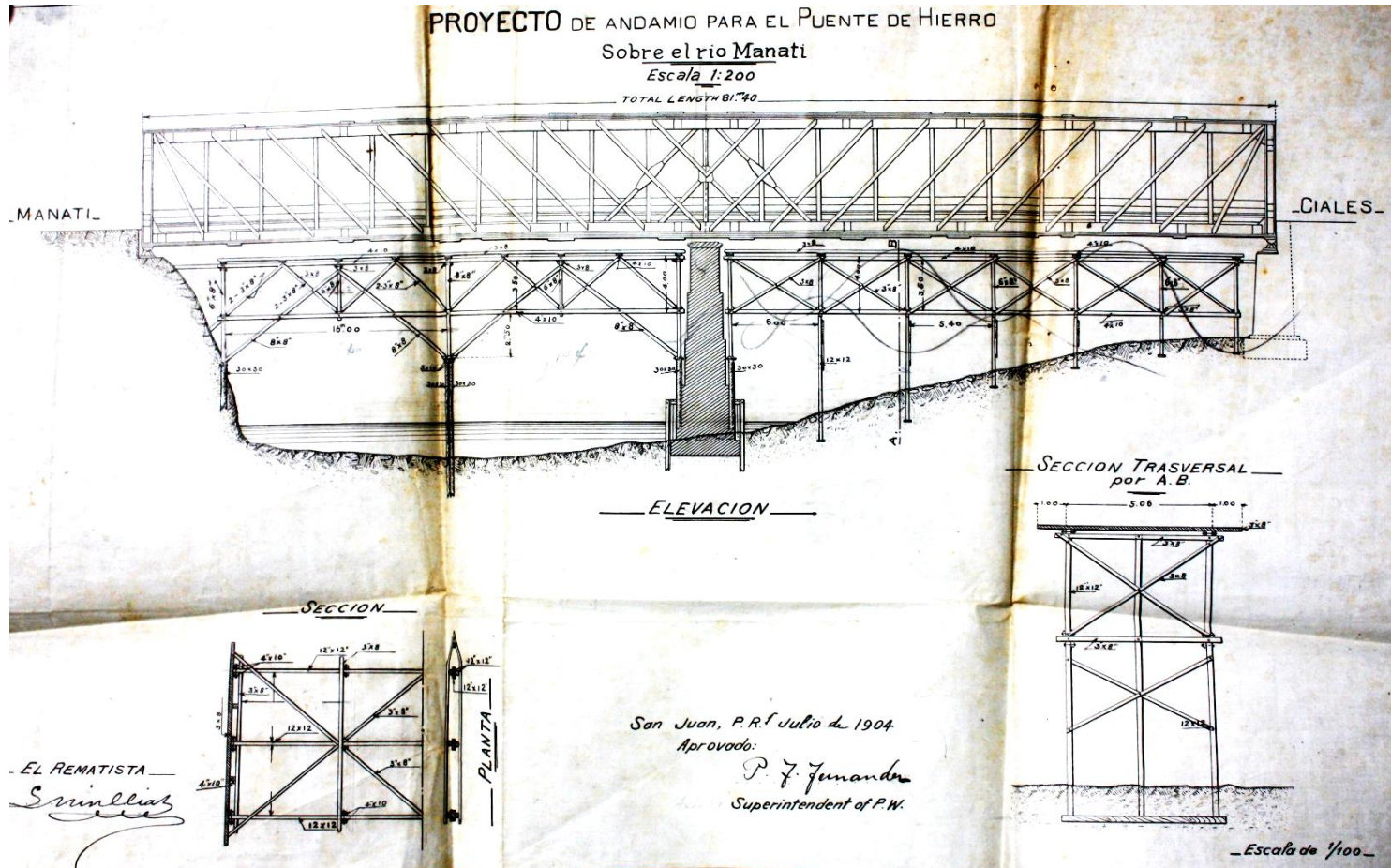
RF = 3.29 (Inventory)
RF = 6.88 (Operating)

Element will need to be
Repair/Rehabilitated

REHABILITATION OF HISTORICAL STEEL BRIDGE NO. 321

REHABILITATION CONSIDERATIONS

□ East & West Truss Diagonals



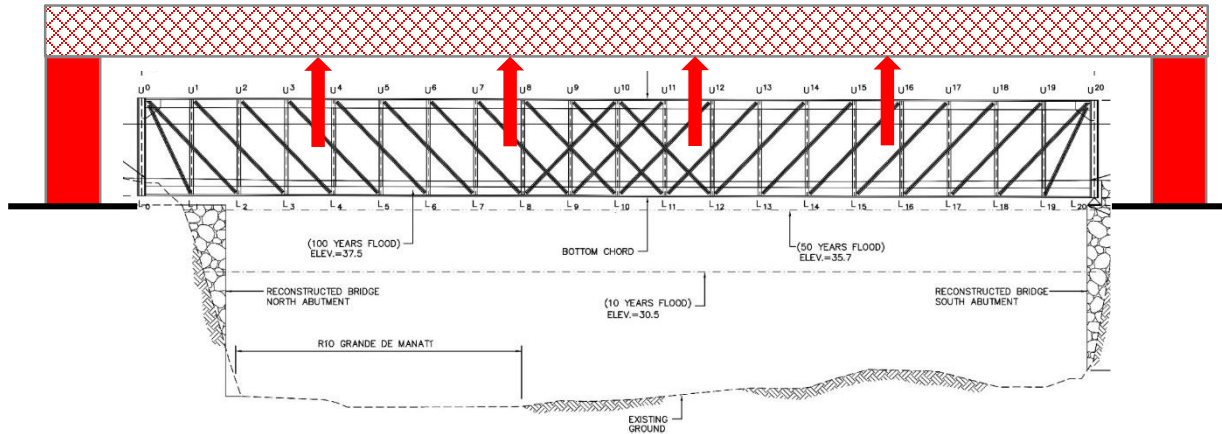
Let's do it the original way...

...temporary support from below?

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REHABILITATION CONSIDERATIONS

❑ East & West Truss Diagonals



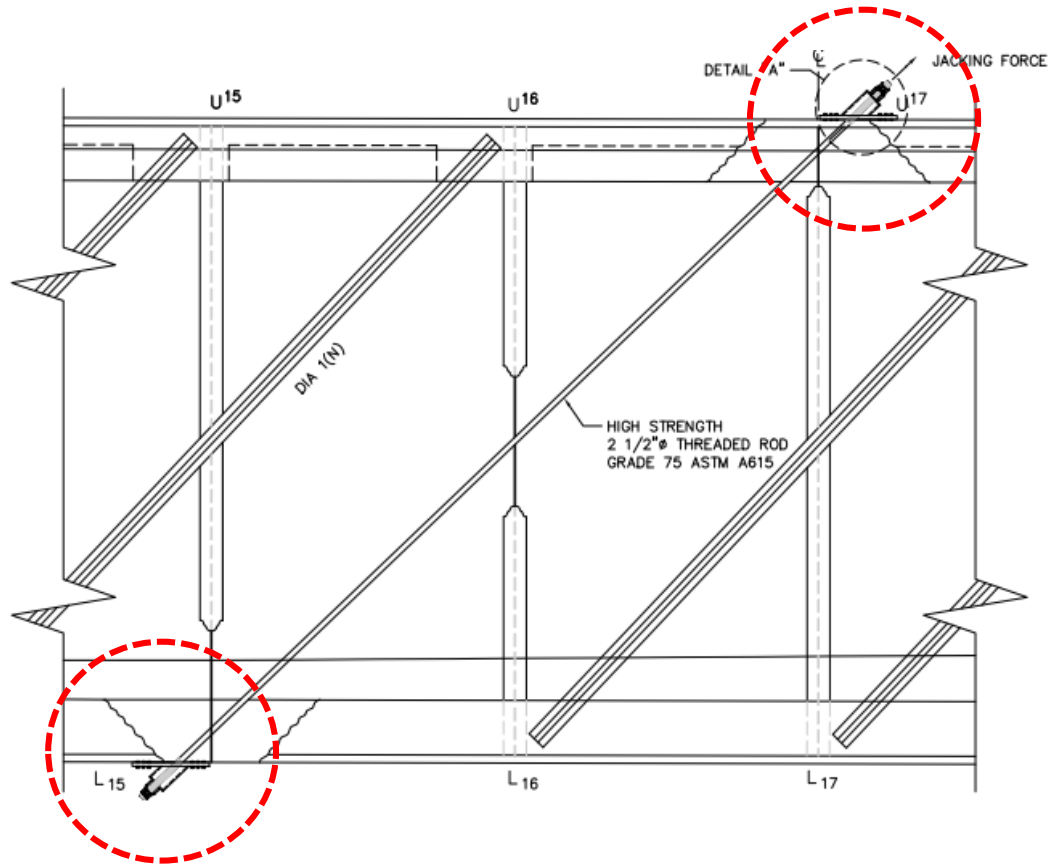
...construction gangway from above?

REHABILITATION OF HISTORICAL STEEL BRIDGE NO. 321

REHABILITATION CONSIDERATIONS

□ East & West Truss Diagonals

...how about a **temporary load transfer mechanism?**



Dead-End Jacking Force System

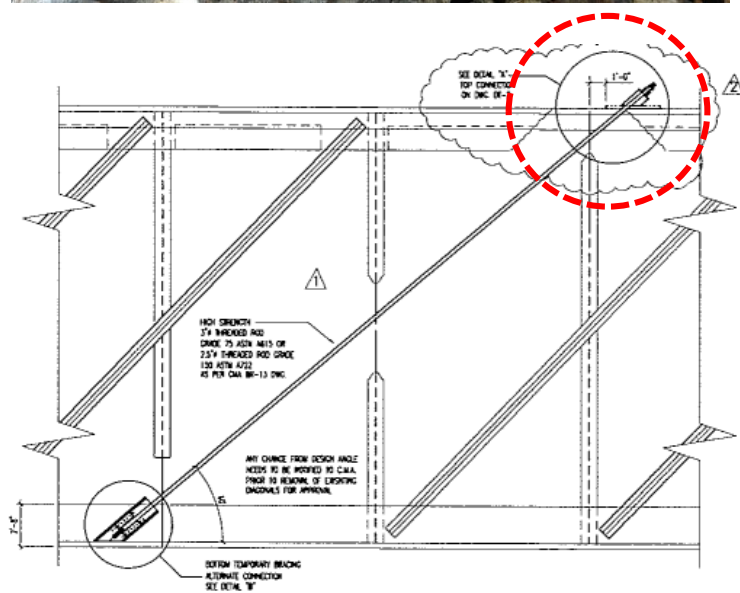
- Readily available equipment (Hydraulic jack, electric pump)
- Area accessibility

Forces will need to be determined with precision

- ✓ 3D State-of-the-Art modeling
- ✓ Knowledge of Const. Live Load

U-11 - L-9	3.35 kips
U-13 - L-11	58.52 kips
U-17 - L-15	142.96 kips
U-19 - L-17	177.90 kips
U-20 - L-18	251.12 kips

REHABILITATION CONSIDERATIONS



Weight = 1400 lbs.

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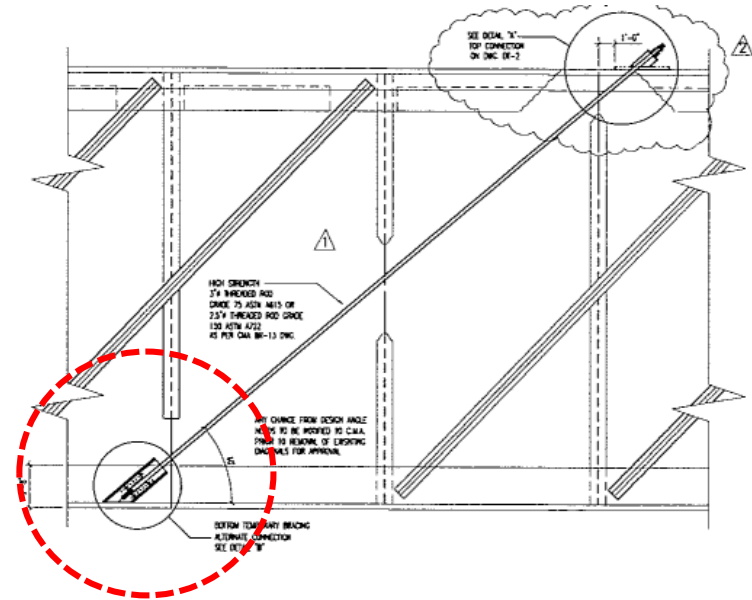
REHABILITATION CONSIDERATIONS

❑ East & West Truss Diagonals



Bottom Chord
Dead End Jack

Weight = 400 lbs.



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REHABILITATION CONSIDERATIONS

❑ East & West Truss Diagonals

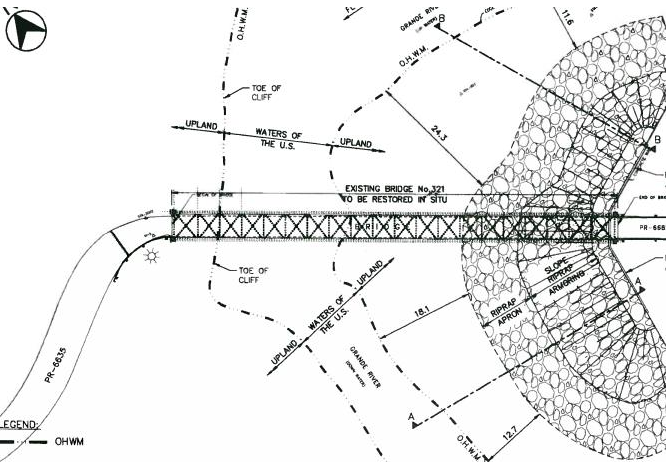


REHABILITATION CONSIDERATIONS

VIDEO

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REHABILITATION CONSIDERATIONS



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REHABILITATION CONSIDERATIONS



REHABILITATION OF HISTORICAL STEEL BRIDGE NO. 321

REHABILITATION CONSIDERATIONS

❑ Rehabilitation Schemes



- Take all superimposed dead weight off the bridge

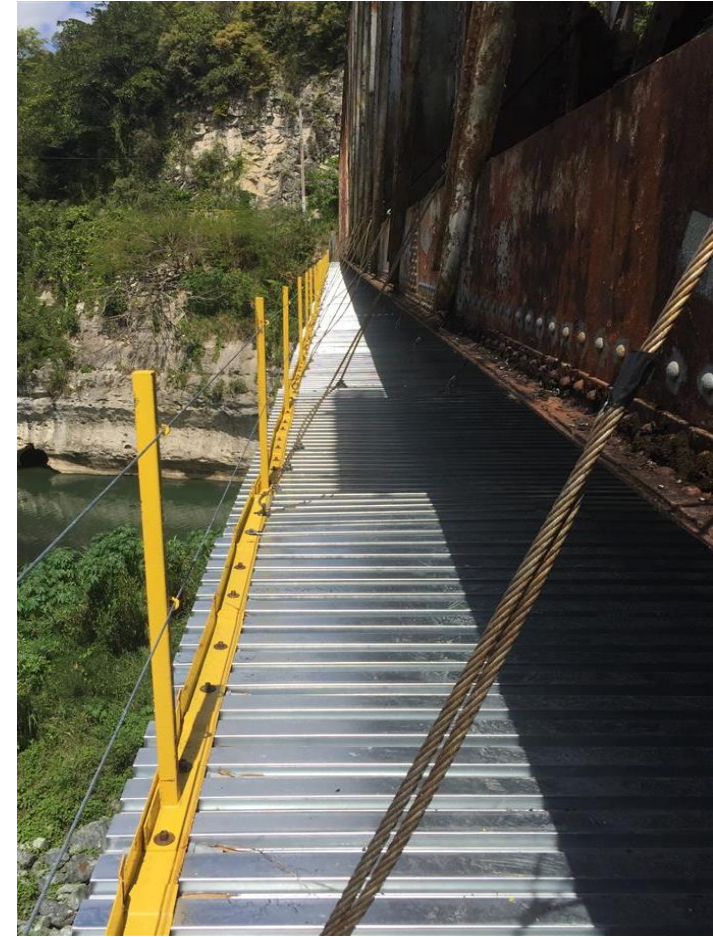
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REHABILITATION CONSIDERATIONS

❑ Rehabilitation Schemes



- **Limit maximum** platform dead load restricted to **10psf**
- **Limit maximum** construction load to **115psf**



REHABILITATION OF HISTORICAL STEEL BRIDGE NO. 321

HIGHLIGHTS

❑ Structural Steel

- Existing Steel **613,000 lbs.**
- Replacement Steel **122,500 lbs.** (20% Existing Steel)
- Added Steel (Capacity Increase) **46,400 lbs.** (8% Existing Steel)

❑ Roadway

- Existing Pavement & Soil **334,000 lbs.**
- New Conc. Slab **417,000 lbs.** (25% Increment)

❑ Diagonal Bracings

- Bracing Replacement Execution Time **16 days** @ Initial stages
- Bracing Replacement Execution Time **5 days** @ later stages
- Steel Plates **A36 (36ksi)**
- Tension Rod (DYWIDAG) **2 1/2" diam. Grade 150** Safety Factor = 3.0
- Hydraulic Pump (ENERPAC) **150 tons**

❑ **Start / Expected Comp.** **Oct. 2013 / Oct. 2017**

❑ **Awarded Cost** **3.3MM**

❑ **Construction Cost** **3.8MM**

HIGHLIGHTS

❑ Key Elements of Success:

- **Collaboration**
 - Between the Owner, Contractor, Inspection and Design Team
 - Proved to be effective in the execution and minimization of environmental risks.
- **Detailed execution strategy plan and protocols**
 - At selective demolition, material selection and rehabilitation work was critical
- **Fabricated materials on-site**
- **Proven execution methodology**
 - for potential rehabilitation work at other bridges with similar structural configurations.

ACKNOWLEDGEMENTS

❑ Collaboration in the design process

- Elvin Pérez - Structural Design
- Jorge Santory – Structural Design
- Mauricio Torres – Bridge Design

❑ Collaboration review process & comments

- Ricardo Herrera
- Jose Torres
- Jose Carro
- Yma Doitteau
- Juan B. Fuentes



Rehabilitation of Historical Steel Bridge No. 321 over Rio Grande de Manatí

